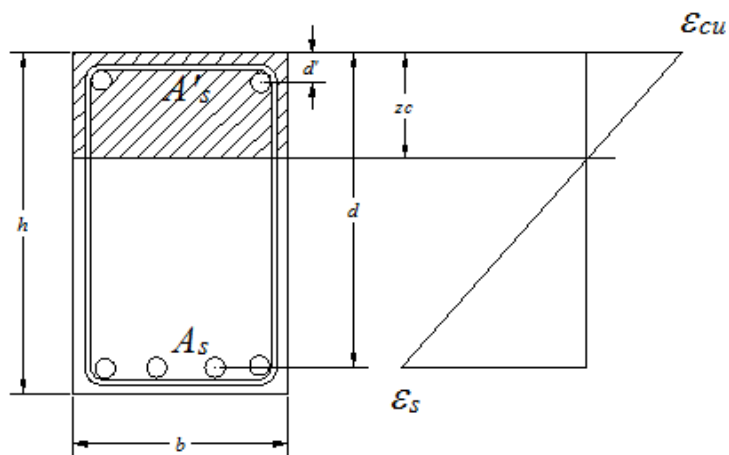
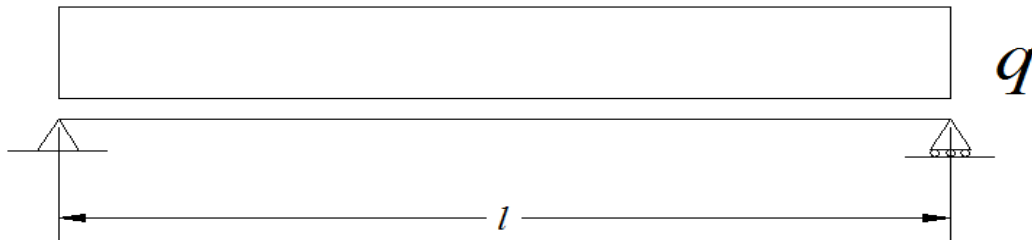


Cemento armato: flessione e taglio, esercizio N3



Per la trave in figura, di luce $l := 4.5 \cdot \text{m}$ soggetta ad un carico di esercizio $q := 40 \cdot \frac{\text{kN}}{\text{m}}$ con una sezione rettangolare di altezza $h := 50 \cdot \text{cm}$ e base $b := 25 \cdot \text{cm}$, armatura tesa di $n_s := 4$ barre $\phi := 20 \cdot \text{mm}$ e armatura compressa di $n_{s1} := 2$ barre $\phi_1 := 16 \cdot \text{mm}$, distanza dai bordi $d_1 := 4 \cdot \text{cm}$, calcolare:

- 1) le sollecitazioni massime,
- 2) le tensioni nel calcestruzzo e nell'acciaio prodotte dal momento massimo,
- 3) il momento ultimo della sezione

Calcestruzzo: $f_{cd} := 18 \cdot \text{MPa}$

Acciaio $f_{yd} := \frac{450}{1.15} \cdot \text{MPa}$ $E_s := 200000 \cdot \text{MPa}$ $n := 15$

$\epsilon_{cu} := 3.5 \cdot 10^{-3}$

Calcolo delle sollecitazioni

$$M_{\text{mx}} := \frac{q \cdot l^2}{8} \quad M_{\text{mx}} = 101.25 \cdot \text{kN} \cdot \text{m}$$
$$V_{\text{mx}} := q \cdot \frac{l}{2} \quad V_{\text{mx}} = 90 \cdot \text{kN}$$

Calcolo delle tensioni

$$A_s := n_s \cdot \pi \cdot \frac{\phi^2}{4} \quad A_s = 12.566 \cdot \text{cm}^2$$
$$A_{s1} := n_{s1} \cdot \pi \cdot \frac{\phi_1^2}{4} \quad A_{s1} = 4.021 \cdot \text{cm}^2$$
$$d := h - d_1 \quad d = 46 \cdot \text{cm}$$
$$A_{st} := A_s + A_{s1} \quad A_{st} = 16.588 \cdot \text{cm}^2$$
$$d_G := \frac{A_s \cdot d + A_{s1} \cdot d_1}{A_{st}} \quad d_G = 35.818 \cdot \text{cm}$$
$$z_c := \left(\sqrt{1 + \frac{2 \cdot b \cdot d_G}{n \cdot A_{st}}} - 1 \right) \cdot \frac{n \cdot A_{st}}{b} \quad z_c = 18.543 \cdot \text{cm}$$
$$J_n := \frac{b \cdot z_c^3}{3} + n \cdot \left[A_{s1} \cdot (z_c - d_1)^2 + A_s \cdot (z_c - d)^2 \right] \quad J_n = 2.08 \times 10^5 \cdot \text{cm}^4$$
$$\sigma_c := \frac{M_{\text{mx}}}{J_n} \cdot z_c \quad \sigma_c = 9.027 \cdot \text{MPa}$$
$$\sigma_s := n \cdot \frac{M_{\text{mx}}}{J_n} \cdot (d - z_c) \quad \sigma_s = 200.485 \cdot \text{MPa}$$

$$\epsilon_y := \frac{f_{yd}}{E_s} \quad \alpha := \frac{\epsilon_{cu}}{\epsilon_y}$$

$$\sigma_s(\epsilon) := \text{if}(|\epsilon| < \epsilon_y, E_s \cdot \epsilon, f_{yd} \cdot \text{signum}(\epsilon))$$

Calcolo del momento ultimo

Percentuale meccanica delle armature

$$\mu_s := A_s \cdot \frac{f_{yd}}{b \cdot d \cdot f_{cd}} \quad \mu_s = 0.238$$

$$\mu_{s1} := \frac{A_{s1} \cdot f_{yd}}{b \cdot d \cdot f_{cd}} \quad \mu_{s1} = 0.076$$

Percentuale meccanica armatura bilanciata

$$K_b := \frac{\epsilon_{cu}}{\epsilon_{cu} + \epsilon_y} \quad K_b = 0.641$$

$$\epsilon_{s1} := \frac{K_b \cdot d - d_1}{K_b \cdot d} \cdot \epsilon_{cu} \quad \epsilon_{s1} = 3.026 \times 10^{-3}$$

$$\mu_{sb} := 0.8 \cdot K_b - \frac{A_{s1} \cdot \sigma_s(\epsilon_{s1})}{b \cdot d \cdot f_{cd}} \quad \mu_{sb} = 0.437$$

$$z_c := d \cdot \frac{(\mu_s - \mu_{s1})}{0.8} \quad z_c = 9.288 \cdot \text{cm}$$

$$\epsilon_{s1} := \epsilon_{cu} \cdot \frac{(z_c - d_1)}{z_c} \quad \epsilon_{s1} = 1.993 \times 10^{-3}$$

$$z_{c2} := d \cdot \frac{\left[\mu_s - \mu_{s1} \cdot \alpha + \sqrt{(\mu_s - \mu_{s1} \cdot \alpha)^2 + 3.2 \cdot \mu_{s1} \cdot \alpha \cdot \frac{d_1}{d}} \right]}{1.6} \quad z_{c2} = 9.229 \cdot \text{cm}$$

$$z_c := \text{if}(\epsilon_{s1} < \epsilon_y, z_{c2}, z_c) \quad z_c = 9.288 \cdot \text{cm}$$

$$\epsilon_{s1} := \epsilon_{cu} \cdot \frac{(z_c - d_1)}{z_c} \quad \epsilon_{s1} = 1.993 \times 10^{-3}$$

$$M_u := A_s \cdot f_{yd} \cdot (d - 0.4 \cdot z_c) + A_{s1} \cdot \sigma_s(\epsilon_{s1}) \cdot (0.4 \cdot z_c - d_1) \quad M_u = 207.478 \cdot \text{kN} \cdot \text{m}$$

